

PRESTANDADEKLARATION

DoP Nr: MKT-540 - sv

¢	Produkttypens unika identifikationskod:	Insprutningssystem VMH för efterföljande rebar anslutning
¢	Avsedd användning/avsedda användningar:	System för efterföljande rebar anslutning, se bilaga/Annex B
¢	Tillverkare:	MKT Metall-Kunststoff-Technik GmbH & Co.KG Auf dem Immel 2 67685 Weilerbach
♦	System för bedömning och fortlöpande kontroll av prestanda:	1
أ	Europeiskt bedömningsdokument:	EAD 330087-00-0601
	Europeisk teknisk bedömning:	ETA-17/0715, 18.07.2018
	Tekniskt bedömningsorgan:	DIBt, Berlin
	Anmält/anmälda organ:	NB 1343 – MPA, Darmstadt

♦ Angiven prestanda:

Väsentliga egenskaper	Prestanda				
Bärförmåga, stadga och beständighet (BWR1)					
Karakteristiska resistanser för statiska och kvasi-statiska belastningar	Bilaga/Annex C1				
Säkerhet vid brand (BWR2)					
Brandegenskaper	Klass A1				
Brandmotstånd	Bilaga/Annex C2 – C3				

Prestandan för ovanstående produkt överensstämmer med den angivna prestandan. Denna prestandadeklaration har utfärdats i enlighet med förordning (EU) nr 305/2011 på eget ansvar av den tillverkare som anges ovan.

Undertecknad på tillverkarens vägnar av:

Stefan Weustenhagen (Verkställande direktör) Weilerbach, 18.07.2018

p.p. Rigelke

Dipl.-Ing. Detlef Bigalke (Produktutveckling direktör)



Originalen av denna deklarationsförklaring skrevs på tyska. Vid avvikelser i översättningen gäller den tyska versionen.

Anchorages subject to:											
Rebar	Ø8	Ø10	Ø12	Ø14	Ø16	Ø 20	Ø22	Ø24	Ø 2 5	Ø 28	Ø3
Static or quasi static action	\checkmark										
Fire exposure						✓					
Tension anchor ZA	M12	2 N	116	M20	M2	4					
Static or quasi static action		\checkmark									
Fire exposure		√									

Base material:

- Reinforced or unreinforced normal weight concrete acc. to EN 206-1:2000
- Strength classes C12/15 to C50/60 acc. to EN 206-1:2000
- Maximum chloride concrete of 0,40 % (CL 0,40) related to the cement content acc. to EN 206-1:2000
- Non-carbonated concrete

Note: In case of a carbonated surface of the existing concrete structure the carbonated layer shall be removed in the area of the post-installed rebar connection with a diameter of \emptyset + 60 mm prior to the installation of the new rebar. The depth of concrete to be removed shall correspond to at least the minimum concrete cover in accordance with EN 1992-1-1:2004+AC:2010.

The foregoing may be neglected if building components are new and not carbonated and if building components are in dry conditions.

Temperature range:

• - 40 °C to +80 °C (max. short term temperature +80 °C and max. long term temperature +50 °C)

Use conditions (Environmental conditions):

- Structures subject to dry internal conditions
 (zinc plated steel, stainless steel or high corrosion resistant steel)
- Structures subject to external atmospheric exposure (including industrial and marine environment) and to permanently damp internal condition, if no particular aggressive conditions exist (stainless steel or high corrosion resistant steel)
- Structures subject to external atmospheric exposure and to permanently damp internal condition, if other particular aggressive conditions exist (high corrosion resistant steel)

Note: Particular aggressive conditions are e.g. permanent, alternating immersion in seawater or the splash zone of seawater, chloride atmosphere of indoor swimming pools or atmosphere with extreme chemical pollution (e.g. in desulphurization plants or road tunnels where de-icing materials are used)

Injection System VHM for rebar connections

Intended use Specifications of intended use

Specifications of intended use

Design:

- · Verifiable calculation notes and drawings are prepared taking account of the loads to be anchored
- Anchorages are designed under the responsibility of an engineer experienced in anchorages and concrete work
- Anchorages are designed in accordance with EN 1992-1-1:2004+AC:2010, EN1992-1-2:2004+AC:2008 and Annex B3 and B4
- The actual position of the reinforcement in the existing structure shall be determined on the basis of the construction documentation and taken into account when designing

Installation:

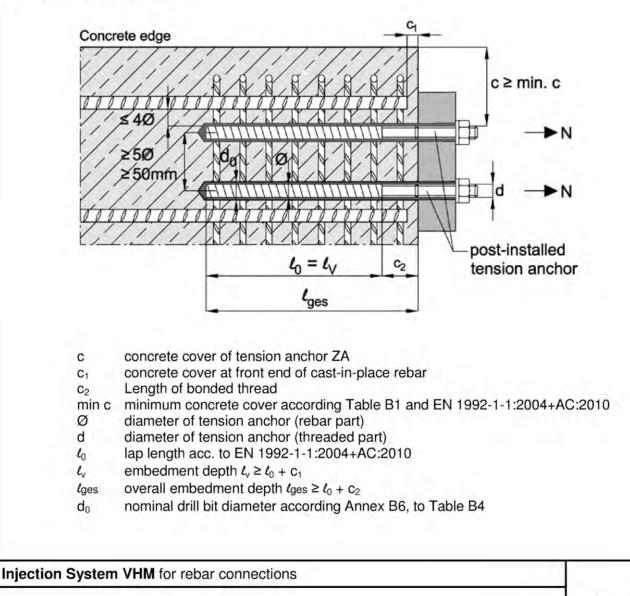
- · Dry or wet concrete
- · Installation in water filled bore holes is not admissible
- · Overhead installation admissible
- · Hole drilling by hammer drill, vacuum drill or compressed air drill
- Check the position of the existing rebars (if the position of existing rebars is not known, it shall be determined using a rebar detector suitable for this purpose as well as on the basis of the construction documentation and then marked on the building component for the overlap joint)
- · The joints for concreting must be roughened to at least such an extent that aggregate protrude
- The installation of post-installed rebar or tension anchor ZA shall be done only by suitable trained installer and under supervision on site; the conditions under which an installer may be considered as suitable trained and the conditions for supervision on site are up to the member states in which the installation is done
- Minimum concrete cover acc. to EN 1992-1-1:2004+AC:2010 must be observed

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General construction rules for tension anchor ZA

- The length for the post-installed thread must not be added to the anchoring length
- · The tension anchor ZA can only transfer forces towards the bar axis
- Tension forces must be transferred by an overlap joint into the present reinforcement of the member
- The transmission of shear forces must be ensured by additional measures, e.g. by shear cleats or anchors with an European Technical Assessment (ETA)
- In the anchor plate the holes for the tension anchors must be executed as elongated holes with axis in the direction of the shear force
- If the clear distance of overlapping bars is greater than 4Ø, the lap length must be increased by a length equal to the clear space where it exceeds 4Ø

Figure B1: Tension Anchor ZA

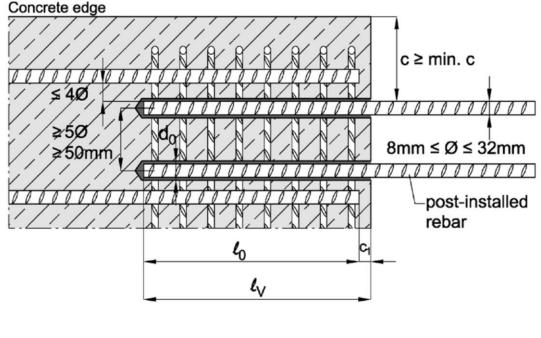


Intended use General construction rules (Tension anchor ZA)

General construction rules for post-installed rebars

- The shear transfer between old and new concrete shall be designed acc. to EN 1992-1-1:2004+AC:2010
- Only tension forces in the axis of the rebar may be transmitted
- If the clear distance of overlapping bars is greater than 4Ø, the lap length must be increased by a length equal to the clear space where it exceeds 4Ø

Figure B2: Post-installed rebars



- c concrete cover of post-installed rebar
- c1 concrete cover at front end of cast-in-place rebar

min c minimum concrete cover according Table B1 and EN 1992-1-1:2004+AC:2010 Ø diameter of tension anchor (rebar)

- l_0 lap length acc. to EN 1992-1-1:2004+AC:2010
- ℓ_v embedment depth $\ell_v \ge \ell_0 + c_1$
- d₀ nominal drill bit diameter according to Annex B6, Table B4

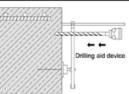
Injection System VHM for rebar connections

Intended use

Annex B4

General construction rules (post-installed rebar)

Table B1:Minimum concrete cover min c1) of post-installed
rebar and tension anchor ZA depending on
drilling method



Drilling method	Rod diameter	min c (without drilling aid device)	min c (<u>with</u> drilling aid device)
Hammer drilling	< 25 mm	$30 \text{ mm} + 0,06 \ \ell_{v} \ge 2 \ \emptyset$	$30 \text{ mm} + 0,02 \ell_{v} \ge 2 \emptyset$
Vacuum drilling	≥ 25 mm	$40 \text{ mm} + 0,06 \ell_v \ge 2 \emptyset$	$40 \text{ mm} + 0,02 \ell_v \ge 2 \emptyset$
Compressed air	< 25 mm	50 mm + 0,08 ℓ_v	50 mm + 0,02 ℓ_v
drilling	≥ 25 mm	60 mm + 0,08 ℓ_v	60 mm + 0,02 <i>l</i> _v

¹⁾ See Annex B3 and B4; Minimum concrete cover acc. to EN 1992-1-1:2004+AC:2010 must be observed

Table B2: Dimensions and installation parameters of tension anchor ZA

Anchor size				M12	M16	M20	M24
Thread diameter			[mm]	12	16	20	24
Rebar diameter			[mm]	12	16	20	25
Cross section area (threaded part)			[mm ²]	84	157	245	353
Width across nut flats	Width across nut flats			19	24	30	36
Effective embedment of	depth	l _v	[mm]	according to static calculation			on
Length of bonded	steel, zinc plated	0	[mm]	≥ 20	≥ 20	≥ 20	≥ 20
thread	A4/HCR	C ₂	[mm]	≥ 100	≥ 100	≥ 100	≥ 100
Maximum installation t	orque	T _{inst}	[Nm]	50	100	150	150

Table B3: Working and curing time

Bore bo	Bore hole temperature		Working time	Minimum curing time		
Bore no			tronking time	dry concrete	wet concrete	
- 5 °C	to	- 1 °C	50 min	5 h	10 h	
0 °C	to	+ 4 °C	25 min	3,5 h	7 h	
+ 5 °C	to	+ 9 °C	15 min	2 h	4 h	
+ 10 °C	to	+ 14 °C	10 min	1 h	2 h	
+ 15 °C	to	+ 19 °C	6 min	40 min	80 min	
+ 20 °C	to	+ 29 °C	3 min	30 min	60 min	
+ 30 °C	to	+ 40 °C	2 min	30 min	60 min	
Cartridg	je ten	nperature		+5°C to +40°C		

Injection System VHM for rebar connections

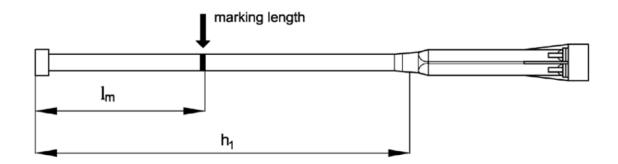
Intended use

Minimum concrete cover / Installation parameters ZA / Working and curing time

	1.	Datil bits all and a second	Brush	Brush diameter		
Rebar Ø	Tension anchor	Drill bit diameter	Brush-Ø	min. Brush-Ø		
Ø	ZA	do	db	d _{b,min}		
[mm]	[-]	[mm]	[mm]	[mm]		
8		12	14	12,5		
10		14	16	14,5		
12	M12	16	18	16,5		
14	1.2.2.6.8	18	20	18,5		
16	M16	20	22	20,5		
20	M20	25	27	25,5		
22		28	30	28,5		
24		32	34	32,5		
25	M24	32	34	32,5		
28		35	37	35,5		
32		40	43	40,5		
ompressed nin. 6 bar) w	vith air valve		commended comp n. 6 bar)			
nin. 6 bar) w	vith air valve		n. 6 bar)	SDS Plus Adapter		
nin. 6 bar) w	vith air valve	(mi	n. 6 bar)	A		
nin. 6 bar) w	vith air valve	(mi	n. 6 bar)	A		
nin. 6 bar) w Srush RB	vith air valve	(mi Brush extension	n. 6 bar)	SDS Plus Adapter		
nin. 6 bar) w	vith air valve	(mi	n. 6 bar)	SDS Plus Adapter		

Rebar	Tension Drill bit				Cart all f	Cartridge: side-by-side (825 ml)				
Ø	anchor ZA	diameter d ₀	Retaining	Retaining	Hand	- or akku-tool	Compre	essed air tool	Compr	essed air too
		G	washer ¹⁾	l _{v,max}	Extension	l _{v,max}	Extension	L v,max	Extension	
[mm]	[-]	[mm]		[cm] pipe		[cm]	pipe	[cm]	pipe	
8		12	-			80		80		
10		14	VM-IA 14		VM-XE 10		VM-XE 10	100	VM-XE 10	
12	M12	16	VM-IA 16	70		100		120		
14		18	VM-IA 18			100		140		
16	M16	20	VM-IA 20					160		
20	M20	25	VM-IA 25			70				
22		28	VM-IA 28		VM-XE 10	70	VM-XE 10	200		
24		32	VM-IA 32	50	VM-XLE 16		VM-XLE 16	200	VM-XLE 16	
25	M24	32	VM-IA 32	50		50				
28		35	VM-IA 35			50		000		
32		40	VM-IA 40					200		

' For horizontal or overhead installation as well as for drill holes deeper than 240mm



On the static mixer and the extension pipe the mortar filling mark l_m and the drill hole depth h₁ must be marked with an adhesive tape or text marker. Rough estimate: $l_m = \frac{1}{3} \cdot h_1$ Fill in the mortar as long until the filling mark l_m will be visible.

Optimal mortar volume: $l_m = h_1 * (1,2 * \frac{\phi^2}{d_0^2} - 0,2)$ [mm]

Length from the end of the retaining washer to the mark on the mixer extension l_m

- drill hole depth = embedment depth ℓ_v resp. ℓ_{aes}) h₁
- Ø rebar diameter
- do nominal drill bit diameter

Injection System VHM for rebar connections

Intended use

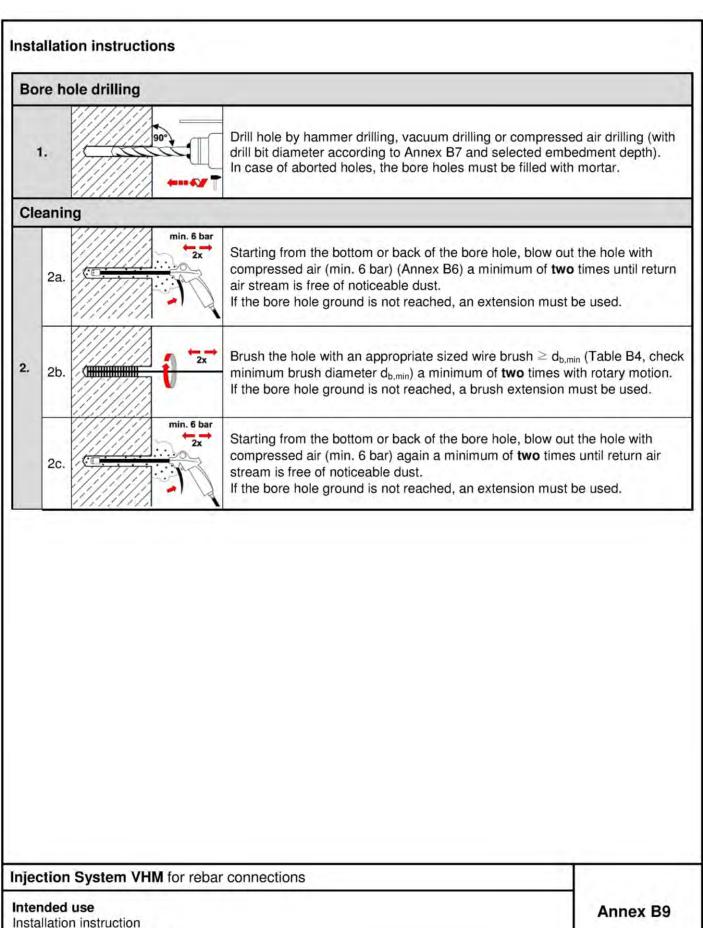
Installation tools, max. embedment depth, marking of extension pipe

	artridge	Hand	Pneumatic tool		
Туре	Size			Theumatic tool	
coaxial	150, 280, 333 ml	e.g.: V	e.g.: VM-P 345 Pneumatic		
соа	380 to 420 ml	e.g.: VM-P 380 Standard	e.g.: VM-P 380 Profi	e.g.: VM-P 380 Pneumatic	
y-side	235, 345 ml	e.g.: VM-P 345 Standard	e.g.: VM-P 345 Profi	e.g.: VM-P 345 Pneumatic	
side-by-side	825 ml	-	-	e.g.: VM-P 825 Pneumatic	

All cartridges can also be extruded by battery tool (e.g. VM-P Akku)

Injection System VHM for rebar connections

Intended use Dispensing tools



Bore hole drilling and cleaning

Prepa	aring and injection	into borehole	
5	e.	Mark the position of the embedment depth ℓ_v on the r	ebar.
6		Check drill hole depth by inserting rebar or anchor ro	d into the empty hole.
7	AND S	Prepare cartridge with static mixer (if necessary with washer). Attach the supplied static-mixing nozzle to cartridge into the correct dispensing tool (Table B6). For every working interruption longer than the (Table B3) as well as for new cartridges, a new static	o the cartridge and load th recommended working tim
″a (X	When extension pipe VM-XLE 16 is used, the tip of t position "X".	he mixer has to be cut off at
8	min.3x →	Prior to applying, discard mortar (forerun) until the mo colour, but at least three full strokes. Never use this r	
9		Fill in injection mortar from the bottom of the clean bo free. Slowly moving the static mixer out of the boreho air inclusions. For embedment larger than 190mm an must be used. Observe the working- and curing time given in table B	ble prevents the formation of extension pipe (Annex B6)
0		For overhead and horizontal installation and embedne retaining washer shall be used. Observe the working- and curing time given in table B	
jectio	on System VHM for	rebar connections	
njectio	on System VHM for ed use	rebar connections	Annex B

nstallation of reb	ar or tension anchor
1	Immediately insert the rebar or tension anchor into the hole while turning slightly until the embedment depth is reached. The bar shall be free of dirt, grease and oil.
2	Excess injection mortar must exceed from the borehole, the annular gap shall be filled completely with mortar. If no mortar exceeds, these requirements are not maintained and the application has to be renewed. For overhead installation fix embedment part (e.g. wedges).
3	Ensure the curing time of the injection mortar according to table B3. Attention: the working time may vary due to different underground temperatures (Table B3). Do not move or load the anchor or rebar until curing time.
4	After the curing time (Table B3) the threaded rod or reinforcing bar can be load.

Minimum anchorage length and minimum lap length

The minimum anchorage length $\ell_{b,min}$ and the minimum lap length $\ell_{0,min}$ according to EN 1992-1-1:2004+AC:2010 ($\ell_{b,min}$ acc. to Eq. 8.6 and Eq. 8.7 and $\ell_{0,min}$ acc. to Eq. 8.11) shall be multiplied by the amplification factor α_{lb} acc. to Table C1.

Table C1: Amplification factor alb depending on concrete strength class and drilling method

Concrete strength class	Drilling method	Rod diameter	Amplification factor α_{lb} [-]
C12/15 to C50/60	hammer drilling vacuum drilling compressed air drilling	Ø8 to Ø32 ZA-M12 to ZA-M24	1,0

Table C2: Reduction factor kb for all drilling methods

Rod diameter		Concrete strength class								
		C12/15	C16/20	C20/25	C25/30	C30/37	C35/45	C40/50	C45/55	C50/60
Ø8 to Ø32 ZA-M12 to ZA-M24	к _ь [-]					1,0				

Table C3: Design values of the ultimate bond stress f_{bd,PIR} in N/mm² for all drilling methods and for good bond conditions

 $f_{bd,PIR} = k_b \cdot f_{bd}$

with

 f_{bd} : Design value of the ultimate bond stress in N/mm² considering the concrete strength classes and the rebar diameter according to EN 1992-1-1:2004+AC:2010 (for all other bond conditions multiply the values by 0,7) k_b : Reduction factor according to Table C2

Rod diameter		Concrete strength class								
		C12/15	C16/20	C20/25	C25/30	C30/37	C35/45	C40/50	C45/55	C50/60
Ø8 to Ø32 ZA-M12 to ZA-M24	f _{bd,PIR} [N/mm²]	1,6	2,0	2,3	2,7	3,0	3,4	3,7	4,0	4,3

Injection System VHM for rebar connections

Design value of ultimate bond stress $f_{bd,fi}$ under fire exposure for concrete classes C12/15 to C50/60 (all drilling methods):

The design value of ultimate bond stress fbd,fi under fire exposure will be calculated by the following equation:

 $f_{bd,fi} = k_{fi}(\theta) \cdot f_{bd,PIR} \cdot \gamma_c / \gamma_{M,fi}$

$$\begin{split} \text{with:} \ \ \theta &\leq 364^{\circ}\text{C} \text{:} \quad k_{\text{fi}}(\theta) = 30,34 \, ^{\star} \, \theta^{(\theta \ast - 0,011)} \, / \, \left(f_{\text{bd},\text{PIR}} \, ^{\star} \, 4,3 \right) \ \leq \ 1,0 \\ \theta &> 364^{\circ}\text{C} \text{:} \quad k_{\text{fi}}(\theta) = 0 \end{split}$$

f _{bd,fi}	design value of ultimate bond stress in case of fire in N/mm ²
θ	Temperature in °C in the mortar layer
$k_{fi}(\theta)$	Reduction factor under fire exposure
f _{bd,PIR}	Design value of the ultimate bond stress in N/mm ² in cold condition according to Table C2 considering concrete class, rebar diameter, drilling method and the bond conditions according to EN 1992-1-1:2004+AC:2010.
γc	partial factor acc. to EN 1992-1-1:2004+AC:2010
YM,fi	partial factor acc. to EN 1992-1-2:2004+AC:2008

For evidence under fire exposure the anchorage length shall be calculated according to EN 1992-1-1:2004+AC:2010 Equation 8.3 using the temperature-dependent ultimate bond stress fbd,fi.

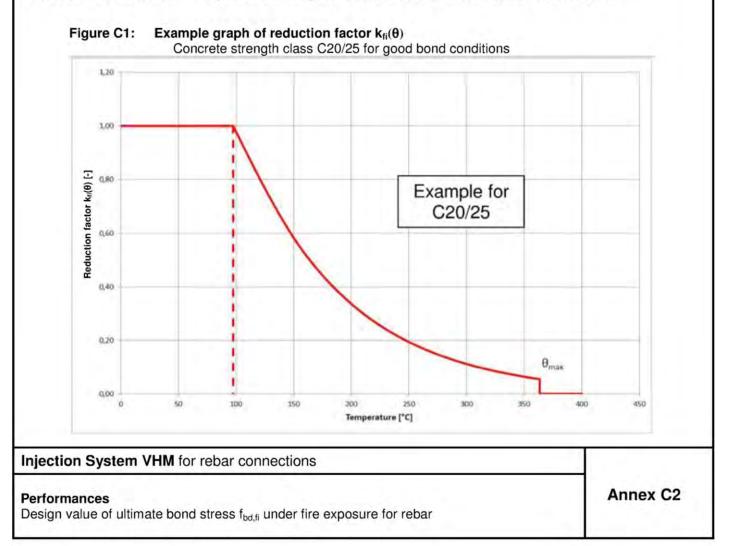


Table C4: Characteristic tension strength in case of fire for tension anchor ZA,

concrete strength class C12/15 to C50/60, acc. to Technical Report TR 020

Tension anchor ZA				M12	M16	M20	M24			
Steel failure										
Steel, zinc plated										
	R30		[N/mm²] -	20						
Characteristic	R60			15						
tension strength	R90	$\sigma_{Rk,s,fi}$		13						
	R120	•			1	0				
Stainless steel A4, HCR										
	R30	$\sigma_{Rk,s,fi}$	[N/mm²] ·		3	0				
Characteristic	R60			25						
tension strength	R90			20						
	R120	•			1	6				

Design value of the tension strength $\sigma_{\text{Rd},\text{s},\text{fi}}$ under fire exposure for tension anchor ZA

The design value of the steel strength $\sigma_{\text{Rd},s,\text{fi}}$ under fire exposure will be calculated by the following equation:

 $\sigma_{\text{Rd,s,fi}} = \sigma_{\text{Rk,s,fi}} / \gamma_{\text{M,fi}}$

with:

 $\begin{array}{ll} \sigma_{\text{Rk},\text{s},\text{fi}} & \text{characteristic steel strength acc. to Table C4} \\ \gamma_{\text{M},\text{fi}} & \text{partial factor under fire exposure acc. to EN 1992-1-2:2004+AC:2008} \end{array}$

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Steel strength for tension anchor ZA under fire exposure